Job Title			Date
Hardwick House, Eastbourne			Aug 2023
Item	Revision	Job No.	Report No.
Drainage Note	B	E8731	RE003

STEPHEN WILSON PARTNERSHIP LTD
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Introduction

This note has been produced for Beford Park Developments by Stephen Wilson Partnership (SWP). The aim of the note is to summarise the outstanding information required raised by East Sussex County Council (ESCC) as the LLFA.

Condition 4

ESCC comments are as follows:

"Condition 4 relates to the detailed design of the surface water drainage system. Whilst detailed drawings have been provided, it was requested as part of the Condition to provide hydraulic calculations. It is noted that these are referenced within the covering email but these do not appear to have been submitted to ourselves as LLFA. We request that the applicant provides hydraulic calculations which correlate with the proposed drawings and is inclusive of hydraulic connectivity of the network. Additionally, evidence that the additional phase of the development has been considered within the network design should be provided."

The drainage calculations proving that the proposed drainage layout is suitable was missed from the planning portal. Please find the calculations as an appendix to this note.

The additional concern raise by ESCC was that the proposed drainage system has considered the additional phase of the development. We can confirm that the proposed drainage system has been designed to accommodate additional phase of the development. The additional phase area has been treated as 100% impermeable for the sake of these calculations.

Additional comments were received following submission of revision A of this note. They are as follows:

"Condition 4 relates to the detailed design of the surface water drainage system. Hydraulic calculations have been provided but impermeable area stated within the document is not consistent. We request the applicant provides a catchment plan shown the impermeable are which is entering the network at which manhole to clarify the design and enable a clear cross-reference between the drawing and calculations."

Following the comments, a drainage catchment area plan has been produced. This can be found in Appendix 2. This shows the impermeable areas being collected by each manhole as shown in the proposed drainage layout.

The error in the hydraulic calculations has been corrected and updated.

Condition 5

ESCC comments are as follows:

"Condition 5 relates to the management and maintenance of the proposed drainage network and, whilst the measures proposed are acceptable the document fails to satisfy part (a) and part (b) of the Condition as it is not made clear who is responsible for undertaking the activities. We request that the applicant provides clear indication of the parties responsible for the maintenance of the drainage system in an updated Management and Maintenance Plan."

ESCC request the applicant provides the details of the parties responsible for the maintenance of the

drainage system.

The onsite drainage will remain private and solely under the ownership of Bedford Park Developments. They will be responsible for the upkeep and maintenance of the existing and proposed drainage on site. A copy of the latest drainage and maintenance plan has been provided to them. The details of Bedford Park Developments are as follows:

Bedford Park Developments
Chalk House Vineyard
Ditchling
East Sussex
BN6 8XB

ben@bedfordparkdevelopments.co.uk

+44 (0) 7393 765 846

Prepared By	Seen By	Date
Craig Searle Civil Engineer	James Maddin Civil Engineers	15-08-2023
Copies to	Ben Ellis (Bedford Park Developments)	

Appendix 1 – Drainage Calculations
Appendix 2 – Drainage Catchment Area

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Title	Hardwick House, Eastbourne	Job No:	E8731
Description :	Surface Water Calculations	By:	CRS
		Date:	Aug-23
		Sheet No:	1

Design Data

M5-60(mm) = 19.600

Ratio R = 0.351

Design Criteria

Design Storm = 100Yr + 45% Climate Change

Discharge Point = Existing private combined drainage

Discharge Rate = 50%+ betterment on existing flows

Total Impermeable Area = 410m^2

Results

Please refer to attached sheets

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Title	Hardwick House, Eastbourne	Job No:	E8731
Decription :	Estimate of Existing and Proposed Peak	By:	CRS
	Run-Off Rate using the Modified Rational Method	Date:	Apr-23
		Sheet No:	2

Existing Site

Pre-Developed Site: Estimate Surface Water Run-Off Using the Modified Rational Method

Site Area = 410 M^2

Existing Impermeable Area = 410 M^2

Average Rate of Rainfall

2 Year 15 Minute Event (M2-15D) = 37.583 mm/hr (i)

30 Year 15 Minute Event (M30-15D) = 71.211 mm/hr (i)

100 Year 15 Minute Event (M100-15D) = 92.206 mm/hr (i)

Average Rainfall Values from FSR Data taken from the MicroDrainage Software

Peak Rate of Run-Off (Q_P)

$Q_P = C \cdot A_P \cdot i$ Where $C = C_V \cdot C_R$

$C_V = 0.75$ (Volumetric Co-efficient)
$C_R = 1.3$ (Routing Co-efficient)

Q_{P2} = 4.173 l/s

Q_{P30} = 7.907 l/s

Q_{P100} = 10.239 l/s

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Title	Hardwick House, Eastbourne	Job No:	E8731
Decription :	Estimate of Existing and Proposed Peak	By:	CRS
	Run-Off Rate using the Modified Rational Method	Date:	Apr-23
		Sheet No:	3

Proposed Development

Total Impermeable Area = 410 M^2

Adjust Average Rainfall for Climate Change in Accordance with the Requirements of the NPPF Technical Guidance, +45% with FSR Data

M2-15D +40% = 54.495 mm/hr (i)

M30-15D +40% = 103.256 mm/hr (i)

M100 - 15D +40% = 133.699 mm/hr (i)

Proposed Peak Rate of Run-Off (Q_P)

Q_{P2} = 6.051 l/s

Q_{P30} = 11.466 l/s

Q_{P100} = 14.846 l/s

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.032	4-8	0.009

Total Area Contributing (ha) = 0.041

Total Pipe Volume (m^3) = 0.462

STORM SEWER DESIGN by the Modified Rational Method

Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	2.962	1.050	2.8	0.004	6.00	0.0	0.600	o	100	Pipe/Conduit	
1.001	12.259	0.155	79.1	0.005	0.00	0.0	0.600	o	100	Pipe/Conduit	
2.000	2.090	0.025	83.6	0.015	6.00	0.0	0.600	o	150	Pipe/Conduit	
1.002	0.500	0.005	100.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
3.000	13.400	0.340	39.4	0.003	6.00	0.0	0.600	o	100	Pipe/Conduit	
1.003	0.650	0.015	43.3	0.014	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.004	3.584	0.065	55.1	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.005	6.600	0.130	50.8	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.00	6.01	5.770	0.004	0.0	0.0	0.0	4.64	36.4	0.5
1.001	50.00	6.25	4.720	0.009	0.0	0.0	0.0	0.87	6.8	1.2
2.000	50.00	6.03	4.590	0.015	0.0	0.0	0.0	1.10	19.4	2.0
1.002	50.00	6.25	4.565	0.024	0.0	0.0	0.0	1.00	17.8	3.2
3.000	50.00	6.18	5.200	0.003	0.0	0.0	0.0	1.23	9.7	0.4
1.003	50.00	6.26	4.560	0.041	0.0	0.0	0.0	1.53	27.1	5.6
1.004	50.00	6.31	4.545	0.041	0.0	0.0	0.0	1.36	24.0	5.6
1.005	50.00	6.38	4.480	0.041	0.0	0.0	0.0	1.42	25.0	5.6

Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam.,L*W (mm)	Pipe Out PN	Pipe Out Invert Level (m)	Pipe Out Diameter (mm)	Pipes In PN	Pipes In Invert Level (m)	Pipes In Diameter (mm)	Backdrop (mm)
RE1	6.370	0.600	Open Manhole	100	1.000	5.770	100				
S1	5.320	0.600	Open Manhole	450	1.001	4.720	100	1.000	4.720	100	
Phase 2	5.900	1.310	Open Manhole	1200	2.000	4.590	150				
S2	5.820	1.255	Open Manhole	600 x 450	1.002	4.565	150	1.001	4.565	100	
								2.000	4.565	150	
RE2	5.900	0.700	Open Manhole	100	3.000	5.200	100				
TANK	5.600	1.040	Junction		1.003	4.560	150	1.002	4.560	150	
								3.000	4.860	100	250
S3	5.600	1.055	Open Manhole	600 x 450	1.004	4.545	150	1.003	4.545	150	
IC7	5.610	1.130	Open Manhole	600 x 450	1.005	4.480	150	1.004	4.480	150	
IC2	5.870	1.520	Open Manhole	1200 x 600		OUTFALL		1.005	4.350	150	

No coordinates have been specified, layout information cannot be produced.

PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	o	100	RE1	6.370	5.770	0.500	Open Manhole	100
1.001	o	100	S1	5.320	4.720	0.500	Open Manhole	450
2.000	o	150	Phase 2	5.900	4.590	1.160	Open Manhole	1200
1.002	o	150	S2	5.820	4.565	1.105	Open Manhole	600 x 450
3.000	o	100	RE2	5.900	5.200	0.600	Open Manhole	100
1.003	o	150	TANK	5.600	4.560	0.890	Junction	
1.004	o	150	S3	5.600	4.545	0.905	Open Manhole	600 x 450
1.005	o	150	IC7	5.610	4.480	0.980	Open Manhole	600 x 450

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	2.962	2.8	S1	5.320	4.720	0.500	Open Manhole	450
1.001	12.259	79.1	S2	5.820	4.565	1.155	Open Manhole	600 x 450
2.000	2.090	83.6	S2	5.820	4.565	1.105	Open Manhole	600 x 450
1.002	0.500	100.0	TANK	5.600	4.560	0.890	Junction	
3.000	13.400	39.4	TANK	5.600	4.860	0.640	Junction	
1.003	0.650	43.3	S3	5.600	4.545	0.905	Open Manhole	600 x 450
1.004	3.584	55.1	IC7	5.610	4.480	0.980	Open Manhole	600 x 450
1.005	6.600	50.8	IC2	5.870	4.350	1.370	Open Manhole	1200 x 600

Online Controls for Storm

Hydro-Brake® Optimum Manhole: S3, DS/PN: 1.004, Volume (m³): 0.3

Unit Reference	MD-SHE-0094-3000-0150-3000
Design Head (m)	0.150
Design Flow (l/s)	3.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	94
Invert Level (m)	4.545
Minimum Outlet Pipe Diameter (mm)	150
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.150	3.0
Flush-Flo™	0.120	3.0
Kick-Flo®	0.145	2.9
Mean Flow over Head Range	-	1.8

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	3.0	1.200	7.9	3.000	12.3	7.000	18.8
0.200	3.4	1.400	8.5	3.500	13.3	7.500	19.5
0.300	4.1	1.600	8.9	4.000	14.2	8.000	20.2
0.400	4.7	1.800	9.5	4.500	15.1	8.500	20.8
0.500	5.2	2.000	10.0	5.000	15.9	9.000	21.4
0.600	5.7	2.200	10.5	5.500	16.7	9.500	22.0
0.800	6.5	2.400	11.0	6.000	17.4		
1.000	7.2	2.600	11.4	6.500	18.2		

Storage Structures for Storm

Complex Manhole: TANK, DS/PN: 1.003

Cellular Storage

Invert Level (m) 4.560 Safety Factor 2.0
Infiltration Coefficient Base (m/hr) 0.00000 Porosity 0.95
Infiltration Coefficient Side (m/hr) 0.00000

Depth (m)	Area (m²)	Inf. Area (m²)	Depth (m)	Area (m²)	Inf. Area (m²)
0.000	26.0	26.0	0.401	0.0	38.0
0.400	26.0	38.0			

Cellular Storage

Invert Level (m) 4.560 Safety Factor 2.0
Infiltration Coefficient Base (m/hr) 0.00000 Porosity 0.95
Infiltration Coefficient Side (m/hr) 0.00000

Depth (m)	Area (m²)	Inf. Area (m²)	Depth (m)	Area (m²)	Inf. Area (m²)
0.000	3.5	3.5	0.401	0.0	7.1
0.400	3.5	7.1			

2 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor	1.000	Additional Flow - % of Total Flow	0.000
Hot Start (mins)	0	MADD Factor * 10m³/ha Storage	2.000
Hot Start Level (mm)	0	Inlet Coeffiecient	0.800
Manhole Headloss Coeff (Global)	0.500	Flow per Person per Day (l/per/day)	0.000
Foul Sewage per hectare (l/s)	0.000		

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Ratio R	0.351
Region	England and Wales	Cv (Summer)	1.000
M5-60 (mm)	19.600	Cv (Winter)	1.000

Margin for Flood Risk Warning (mm)	150.0
Analysis Timestep	2.5 Second Increment (Extended)
DTS Status	ON
DVD Status	ON
Inertia Status	ON

Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 240, 360, 480, 960, 1440
Return Period(s) (years)	2, 30, 100
Climate Change (%)	0, 0, 45

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)
1.000	RE1	15 Summer	2	+0%					5.781
1.001	S1	15 Summer	2	+0%	100/15 Summer				4.755
2.000	Phase 2	30 Summer	2	+0%	30/30 Summer				4.655
1.002	S2	30 Summer	2	+0%	30/15 Summer				4.654
3.000	RE2	15 Summer	2	+0%					5.216
1.003	TANK	30 Summer	2	+0%	30/15 Summer				4.654
1.004	S3	30 Summer	2	+0%	30/15 Winter				4.673
1.005	IC7	60 Summer	2	+0%					4.517

PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.000	RE1	-0.089	0.000	0.03			0.8	OK	
1.001	S1	-0.065	0.000	0.26			1.7	OK	
2.000	Phase 2	-0.085	0.000	0.25			2.8	OK	
1.002	S2	-0.061	0.000	0.40			4.3	OK	
3.000	RE2	-0.084	0.000	0.06			0.6	OK	

2 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m^3)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.003	TANK	-0.056	0.000	0.27		18	3.0	OK*	
1.004	S3	-0.022	0.000	0.18			2.8	OK	
1.005	IC7	-0.113	0.000	0.14			2.9	OK	

30 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor	1.000	Additional Flow - % of Total Flow	0.000
Hot Start (mins)	0	MADD Factor * 10m³/ha Storage	2.000
Hot Start Level (mm)	0	Inlet Coeffiecient	0.800
Manhole Headloss Coeff (Global)	0.500	Flow per Person per Day (l/per/day)	0.000
Foul Sewage per hectare (l/s)	0.000		

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Ratio R	0.351
Region	England and Wales	Cv (Summer)	1.000
M5-60 (mm)	19.600	Cv (Winter)	1.000

Margin for Flood Risk Warning (mm)	150.0
Analysis Timestep	2.5 Second Increment (Extended)
DTS Status	ON
DVD Status	ON
Inertia Status	ON

Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 240, 360, 480, 960, 1440
Return Period(s) (years)	2, 30, 100
Climate Change (%)	0, 0, 45

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)
1.000	RE1	15 Summer	30	+0%					5.784
1.001	S1	15 Summer	30	+0%	100/15 Summer				4.775
2.000	Phase 2	60 Summer	30	+0%	30/30 Summer				4.767
1.002	S2	60 Summer	30	+0%	30/15 Summer				4.766
3.000	RE2	15 Summer	30	+0%					5.223
1.003	TANK	60 Summer	30	+0%	30/15 Summer				4.765
1.004	S3	30 Winter	30	+0%	30/15 Winter				4.795
1.005	IC7	60 Summer	30	+0%					4.521

PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.000	RE1	-0.086	0.000	0.05			1.5	OK	
1.001	S1	-0.045	0.000	0.58			3.7	OK	
2.000	Phase 2	0.027	0.000	0.35			3.8	SURCHARGED	
1.002	S2	0.051	0.000	0.56			6.1	SURCHARGED	
3.000	RE2	-0.077	0.000	0.12			1.1	OK	

30 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m^3)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.003	TANK	0.055	0.000	0.32		30	3.5	SURCHARGED*	
1.004	S3	0.100	0.000	0.21			3.4	SURCHARGED	
1.005	IC7	-0.109	0.000	0.16			3.4	OK	

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor	1.000	Additional Flow - % of Total Flow	0.000
Hot Start (mins)	0	MADD Factor * 10m³/ha Storage	2.000
Hot Start Level (mm)	0	Inlet Coeffiecient	0.800
Manhole Headloss Coeff (Global)	0.500	Flow per Person per Day (l/per/day)	0.000
Foul Sewage per hectare (l/s)	0.000		

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model		FSR	Ratio R 0.351
Region	England and Wales	Cv (Summer)	1.000
M5-60 (mm)	19.600	Cv (Winter)	1.000

Margin for Flood Risk Warning (mm)	150.0
Analysis Timestep	2.5 Second Increment (Extended)
DTS Status	ON
DVD Status	ON
Inertia Status	ON

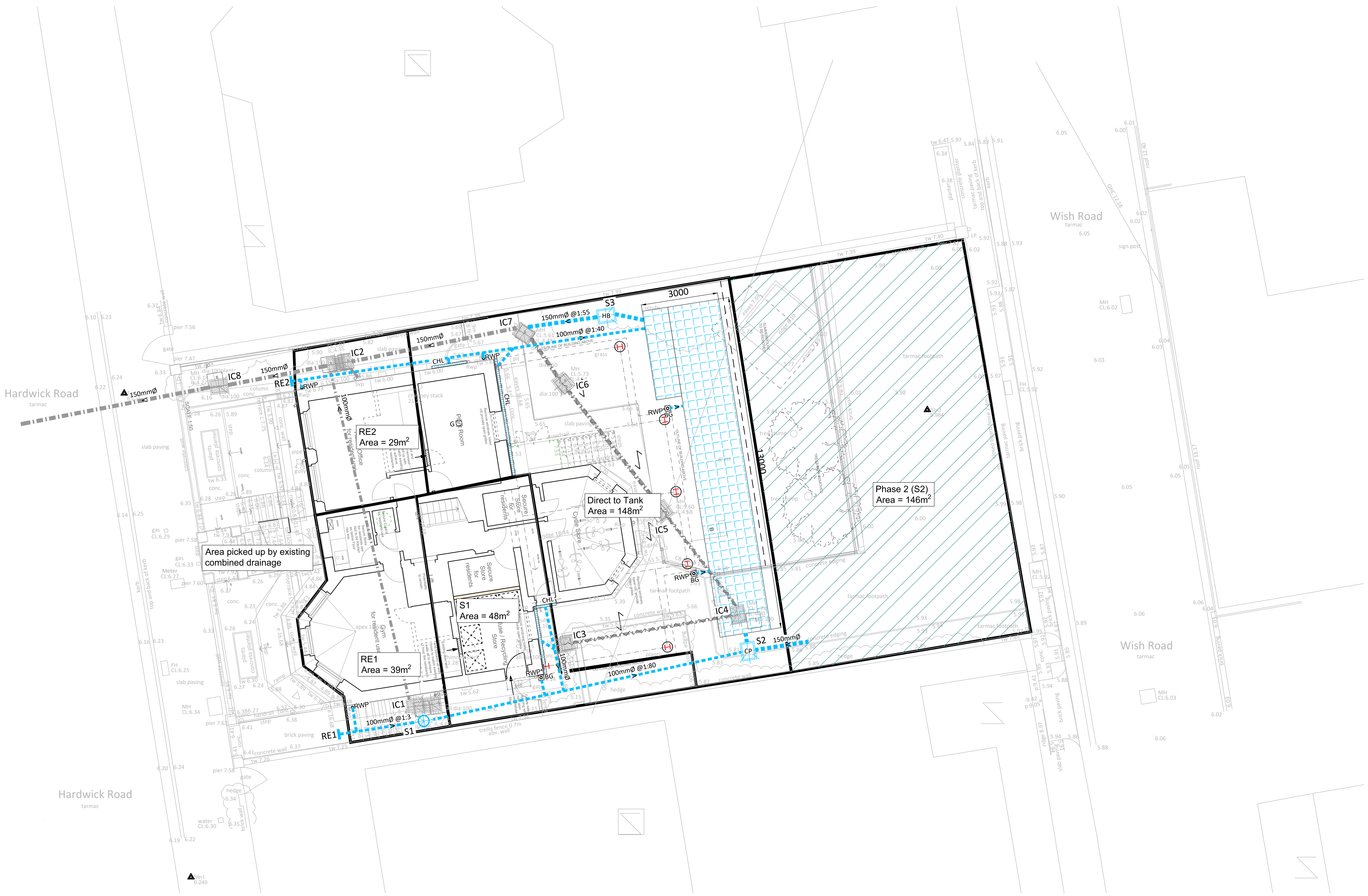
Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 240, 360, 480, 960, 1440
Return Period(s) (years)	2, 30, 100
Climate Change (%)	0, 0, 45

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)
1.000	RE1	15 Summer	100	+45%					5.791
1.001	S1	60 Summer	100	+45%	100/15 Summer				5.217
2.000	Phase 2	60 Summer	100	+45%	30/30 Summer				5.207
1.002	S2	60 Summer	100	+45%	30/15 Summer				5.204
3.000	RE2	15 Summer	100	+45%					5.232
1.003	TANK	60 Summer	100	+45%	30/15 Summer				5.203
1.004	S3	60 Summer	100	+45%	30/15 Winter				5.201
1.005	IC7	60 Summer	100	+45%					4.534

PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.000	RE1	-0.079	0.000	0.10			2.7	OK	
1.001	S1	0.397	0.000	0.72			4.6	FLOOD RISK	
2.000	Phase 2	0.467	0.000	0.65			7.1	SURCHARGED	
1.002	S2	0.489	0.000	1.06			11.5	SURCHARGED	
3.000	RE2	-0.068	0.000	0.22			2.1	OK	

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m^3)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.003	TANK	0.493	0.000	0.55		43	5.9	SURCHARGED*	
1.004	S3	0.506	0.000	0.37			5.9	SURCHARGED	
1.005	IC7	-0.096	0.000	0.28			5.9	OK	



Drawing Legend	
Drainage Catchment Areas	
[hatched box]	Phase 2 development area
[bold box]	Drainage Catchment Area. Size and manhole as noted.
Drainage Layout	
[grey dashed line]	Existing Surface Water Drainage
[grey zigzag line]	Existing Surface Water Drainage to be removed
[blue dashed line]	Proposed Surface Water Drainage
[symbol]	Type D inspection chamber Flexible Construction
[symbol]	Type D inspection chamber Rigid Construction
RE	Rodding Eye
[symbol]	Geocellular attenuation tank
RWP RWP*	Rainwater downpipe location. * denotes discharge to gully
BG	Bottle Gully
CHL	Channel Drain
CP	Catchpit
HB	Hydrobrake by Hydro International

NOTES:

1. This drawing is to be read in conjunction with all other SWP drawings, and with all relevant architect's and engineer's drawings and specification and any discrepancies found are to be reported immediately to the engineer.
2. No dimensions are to be scaled from this drawing. unless noted otherwise all dimensions are in millimeters and all levels are in metres from the site datum.
3. All dimensions to be checked on site. All details and dimensions relating to sub-contractors work must be checked and agreed between the sub-contractor or supplier and the general contractor.
4. The electronic information from this drawing can not be guaranteed as dimensionally drawn exact. figured dimensions must be used for setting out and detailing. swp logos and company information must be removed from copies if information is re-used.
5. The main contractor is responsible for the design of all temporary works, and is also responsible for the safe maintenance and stability of existing buildings at all times.
6. The main contractor is responsible for all occurrences of ground water during the construction period.
7. Any information given regarding existing underground services is given in good faith after consultation with the relevant authority, however accuracy is not certain. The main contractor is responsible for checking all information on site prior to work commencing and taking due care and attention whilst undertaking the works.
8. The contractor must comply with all current legislation relating to health & safety.
9. All products specified shall be installed in strict accordance with the manufacturers recommendations and instructions. If there are discrepancies between that information and the details on any swp drawings, the manufacturers instructions must be used.

Proposed Foul Drainage not shown for clarity.

COLOURED DRAWING
REQUEST A COLOURED COPY IF THIS STAMP IS NOT RED

1m 2m 3m 4m 5m 10m
SCALE 1-100

FOR INFORMATION ONLY

REV.	DATE	DESCRIPTION
P1	15.08.2023	PRELIMINARY ISSUE

CLIENT: BEDFORD PARK DEVELOPMENTS

ARCHITECT: GARRICK ARCHITECTS

JOB TITLE: 6 HARDWICK HOUSE, HARDWICK ROAD EASTBOURNE, BN21 4NY

DRAWING TITLE: DRAINAGE CATCHMENT PLAN

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SCALE AT A1	DATE	DRAWN	JOB No.	DRAWING No.	REV.
1-100	AUGUST 2023	CRS	E8731	202	P1
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